www.EquipmentAnchorage.com

PATTERSON-KELLEY CO.

DURATION III® WATER HEATER

DES. J. ROBERSON

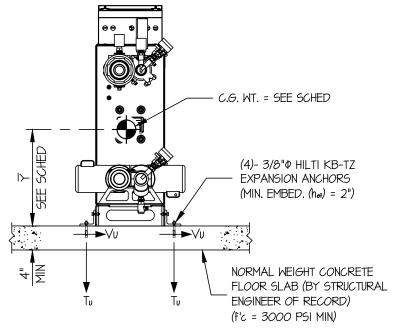
JOB NO. 11-1707

DATE 3/21/17

SHEET 1

SHEETS

SEISMIC ANCHORAGE SLAB ON GRADE



FRONT ELEVATION

NOTES:

1. FORCES ARE DETERMINED PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10

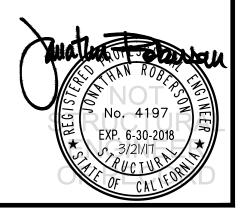
STRENGTH DESIGN IS USED. (SDS = 2.20, Δp = 1.0, |p| = 1.5, Rp = 2.5, Ω_0 = 2.0, z/h = 0)

HORIZONTAL FORCE (Eh) = 0.99 Wp

HORIZONTAL FORCE (Emh) = 1.98 Wp (FOR CONCRETE ANCHORAGE)

VERTICAL FORCE (Ev) = 0.44 Wp

- 2. CENTER OF GRAVITY (C.G.) AND WEIGHT ARE THE GOVERNING PARAMETERS FOR DESIGN. THESE CALCULATIONS ENCOMPASS ALL WEIGHTS UP TO THE MAXIMUM WEIGHT SHOWN.
- 3. STRUCTURAL ENGINEER OF RECORD FOR THE BUILDING SHALL PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.



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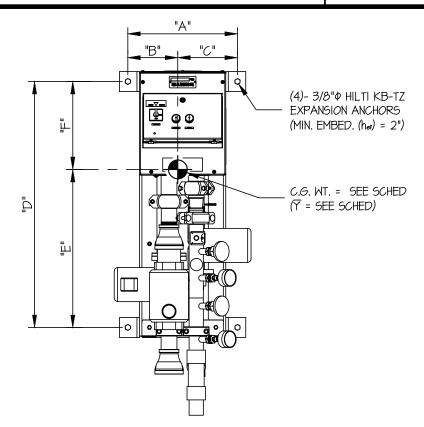
11-1707 JOB NO.

3/21/17 DATE

SHEET

SHEETS SLAB ON GRADE

SEISMIC ANCHORAGE



PLAN AT BASE

UNIT	WEIGHT (lb.)	\rightarrow{\rightarrow{\text{\text{r.}}}}{\text{(in.)}}	"A" (in.)	B" (in.)	"C" (in.)	(in.)	"E" (in.)	"F" (in.)	Tu (lb.)	Vu (lb.)
PUMPED	475	18.2	16.5	7.5	9	37	23.8	13.2	666	379
INSTANTANEOUS	500	18.6	16.5	7.6	8.9	37	23.9	13.1	721	400

LOADS: PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10.

STRENGTH DESIGN IS USED (SDS = 2.20, Δp = 1.0, |p| = 1.5, Rp = 2.5, Ω_0 = 2.0, z/h = 0)

WEIGHT = 500 LB

HORIZONTAL FORCE (Emh) = 1.98 Wp = 990 LB

VERTICAL FORCE (E_v) = 0.44 W_p = 220 LB

BOLT FORCES:

BOLT SPECS: 3/8" # HILTI KB-TZ (hef = 2") $\phi T = 0.75 \phi Nn = 1212 LB/BOLT (TENSION)$

 $\phi V = \phi V n = 1466 LB/BOLT$ (SHEAR)

TENSION (T)

$$T_{\text{U MAXIMUM}} = \left[\frac{990 \# (18.6'')(7.6'')}{1 \text{ BOLT } (37'')(16.5'')} \times (0.3) \right] + \frac{990 \# (18.6'')(23.9'')}{1 \text{ BOLT } (16.5'')(37'')} - \frac{(500 \# (0.9) - 220 \#)(7.6'')(23.9'')}{1 \text{ BOLT } (16.5'')(37'')} = 721 \text{ LB/BOLT } (\text{MAX})$$

$$(\text{HORIZ - FRONT TO BACK}) \qquad (\text{HORIZ - SIDE TO SIDE}) \qquad (\text{WBGHT(0.9) - Ev})$$

SHEAR (V)

$$V_{u \text{ MAXIMUM}} = \left(\frac{990\#(8.9")}{2 \text{ BOLTS} (16.5")} \times (0.3)\right) + \frac{990\#(23.9")}{2 \text{ BOLTS} (37")} = 400 \text{ LB/BOLT (MAX)}$$

UNITY CHECK:

$$\left(\begin{array}{c} T\, \underline{U} \\ \hline \phi T \end{array}\right) \,+\, \left(\begin{array}{c} V\, \underline{U} \\ \hline \phi V \end{array}\right) \,\,\leq\,\, 12 \quad \left(\begin{array}{c} \underline{721} \\ \underline{1212} \end{array}\right) \,+\, \left(\begin{array}{c} \underline{400} \\ \underline{1466} \end{array}\right) \,\,=\,\, 0.87 \,\,\leq\,\, 12 \quad . \,\, \stackrel{\textstyle \circ}{.} \quad \, \, \underline{O.K.}$$

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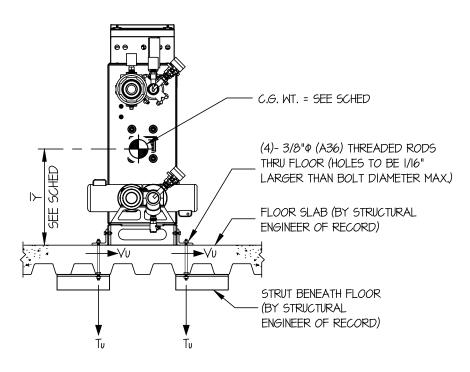
DATE 3/21/17

SHEET 1

SHEETS

SEISMIC ANCHORAGE

<u>UPPER FLOOR</u>



FRONT ELEVATION

NOTES:

1. FORCES ARE DETERMINED PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10.

STRENGTH DESIGN IS USED. (Sps = 2.20, 2p = 1.0, p = 1.5, p = 2.5, z/h < 1)

HORIZONTAL FORCE (Eh) = 1.58 Wp VERTICAL FORCE (Ev) = 0.44 Wp

- 2. CENTER OF GRAVITY (C.G.) AND WEIGHT ARE THE GOVERNING PARAMETERS FOR DESIGN. THESE CALCULATIONS ENCOMPASS ALL WEIGHTS UP TO THE MAXIMUM WEIGHT SHOWN.
- 3. STRUCTURAL ENGINEER OF RECORD FOR THE BUILDING SHALL PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.



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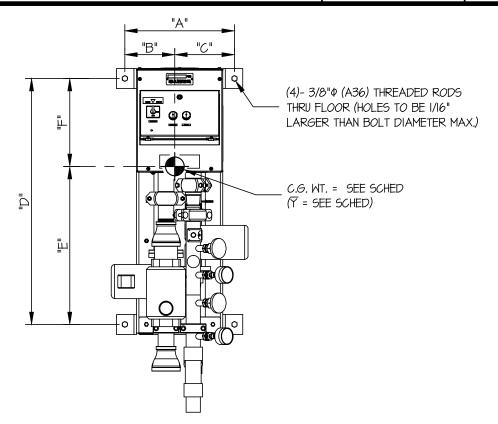
11-1707 JOB NO.

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SHEET

SHEETS UPPER FLOOR

SEISMIC ANCHORAGE



PLAN AT BASE

UNIT	WEIGHT (lb.)	\rightarrow{\righ	"A" (in.)	B" (in.)	"C" (in.)	"D" (in.)	"E" (in.)	"F" (in.)	Tu (lb.)	Vu (lb.)
PUMPED	475	18.2	16.5	7.5	9	37	23.8	13.2	519	303
INSTANTANEOUS	500	18.6	16.5	7.6	8.9	37	23.9	13.1	562	319

LOADS: PER 2016 CALIFORNIA BUILDING CODE AND ASCE 7-10.

STRENGTH DESIGN IS USED (SDS = 2.20, 2p = 1.0, 1p = 1.5, 1p = 2.5, 1p z/h < 1p

WEIGHT = 500 LB

HORIZONTAL FORCE (En) = 1.58 Wp = 790 LB

VERTICAL FORCE (E_V) = 0.44 W_p = 220 LB

BOLT FORCES:

BOLT SPECS: 3/8"ø (A36) THREADED ROD

φT= 3589 LB/BOLT (TENSION)

ΦV= 1914 LB/BOLT (SHEAR)

TENSION (T)

$$T_{\text{u MAXIMUM}} = \left[\frac{790 \# (18.6'')(7.6'')}{1 \text{ BOLT } (37'')(16.5'')} \times (0.3) \right] + \frac{790 \# (18.6'')(23.9'')}{1 \text{ BOLT } (16.5'')(37'')} - \frac{(500 \# (0.9) - 220 \#)(7.6'')(23.9'')}{1 \text{ BOLT } (16.5'')(37'')} = 562 \text{ LB/BOLT } (\text{MAX})$$

$$(\text{HORIZ - FRONT TO BACK}) \qquad (\text{HORIZ - SIDE TO SIDE}) \qquad (\text{WEIGHT(0.9) - Ev})$$

SHEAR (V)

$$V_{u \text{ MAXIMUM}} = \left(\frac{790 \# (8.9'')}{2 \text{ BOLTS} (16.5'')} \times (0.3)\right) + \frac{790 \# (23.9'')}{2 \text{ BOLTS} (37'')} = 319 \text{ LB/BOLT (MAX)}$$