www.EquipmentAnchorage.com

PATTERSON KELLEY

STORM MODELS 2500/3000 & 3500/4000 CONDENSING BOILERS

DES. J. ROBERSON

2/15/23

JOB NO. 11-2304

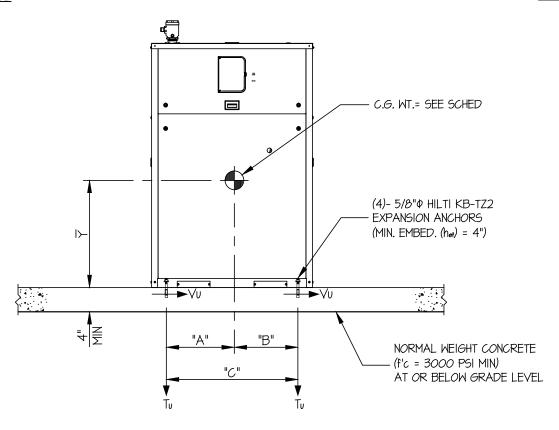
DATE

SHEET

_F 2 _{SHEETS}

SEISMIC ANCHORAGE

SLAB ON GRADE



FRONT ELEVATION

NOTES:

1. FORCES ARE DETERMINED PER 2022 CALIFORNIA BUILDING CODE AND ASCE 7-16. STRENGTH DESIGN IS USED. (EXAMPLE: SDS = 2.30, 2p = 1.0, 2p = 1.5, 2p = 2.5, 2p = 2.0, 2p = 0.

HORIZONTAL FORCE (Eh) = 1.035 Wp

HORIZONTAL FORCE (Emh) = 2.07 Wp (FOR CONCRETE ANCHORAGE)

VERTICAL FORCE (Ev) = 0.46 Wp

- 2. THIS CALCULATION ENCOMPASSES WEIGHTS AND VERTICAL C.G. POSITIONS NOT EXCEEDING VALUES SHOWN.
- 3. THIS CALCULATION WAS PREPARED WITHOUT KNOWLEDGE OF ANY SITE CONDITION. COMPATIBILITY FOR USE WITH A SITE SHALL BE EVALUATED BY THE STRUCTURAL ENGINEER OF RECORD OF THE INSTALLATION (SEOR). USE REQUIRES APPROVAL BY THE SEOR.
- 4. STRUCTURAL ENGINEER OF RECORD FOR THE INSTALLATION SHALL VERIFY ALL CONDITIONS, EVALUATE INTERACTION WITH ADJACENT EQUIPMENT AND ANCHORS, AND PROVIDE SUPPORT STRUCTURE DESIGNED TO SUPPORT WEIGHTS AND FORCES SHOWN IN COMBINATION WITH ALL OTHER LOADS THAT MAY BE PRESENT.



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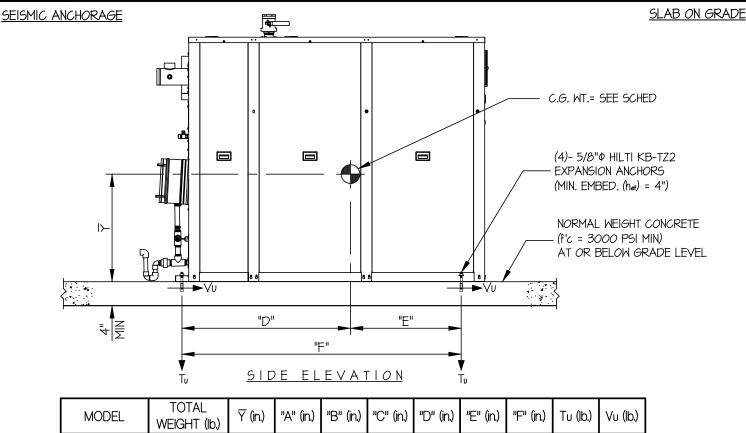
DES. J. ROBERSON

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DATE 2/15/23

2

2 SHEETS



MODEL	TOTAL WEIGHT (lb.)	√ (in.)	"A" (in.)	"B" (in.)	"C" (in.)	"D" (in.)	"E" (in.)	"F" (in.)	Tu (lb.)	Vu (lb.)
2500/3000	2329	29.64	18.31	17.69	36	32.56	28.19	60.75	2207	1660
* 3500/4000	2772	29.28	18.72	17.28	36	41.26	35.73	76.99	2502	1985

^{*} THIS UNIT IS USED IN CALCULATIONS

LOADS:

WEIGHT (W_p) = 2772 LB

HORIZONTAL FORCE (Emh) = 2.07 Wp = 5738 LB

VERTICAL FORCE (Ev) = 0.46 Wp = 1275 LB

ANCHOR FORCES:

TENSION (T)

 $TU \text{ MAXIMUM} = \left[\frac{5738\#(29.28'')(17.28'')}{1 \text{ BOLT} (76.99'')(36'')} \times (0.3) \right] + \frac{5738\#(29.28'')(41.26'')}{1 \text{ BOLT} (36'')(76.99'')} - \frac{(2772\#(0.9) - 1275\#)(17.28'')(41.26'')}{1 \text{ BOLT} (36'')(76.99'')} = 2502 \text{ LB/BOLT (MAX)}$

(HORIZ - FRONT TO BACK)

(HORIZ. - SIDE TO SIDE)

(0.9(WEIGHT) - Ev)

ANCHOR SPEC: 3/8" P HILTI KB-TZ2: (hef = 4")

= 3632 LB/ANCHOR (TENSION)

= 6668 LB/ANCHOR (SHEAR)

SPACING = 14" MIN

EDGE DISTANCE = 26"MIN:

 $\phi T = 0.75 \phi Nn$

 $\phi V = \phi V n$

SHEAR (V)

Vu maximum =
$$\left[\frac{5738\#(18.72")}{2 \text{ BoLTS}(36")} \times (0.3)\right] + \frac{5738\#(41.26")}{2 \text{ BOLTS}(76.99")} = 1985 \text{ LB/BOLT (MAX)}$$

UNITY CHECK:

$$\left(\frac{T_u}{\Phi T}\right) + \left(\frac{V_u}{\Phi V}\right) \le 12 \quad \left(\frac{2502}{3632}\right) + \left(\frac{1985}{6668}\right) = 0.99 \le 12$$
 ... O.K.

2/15/23

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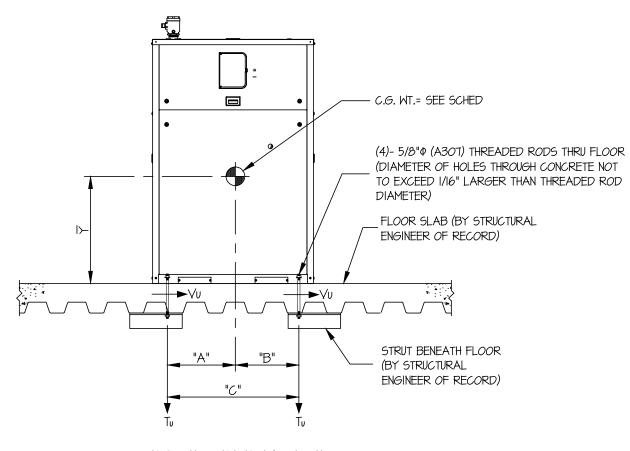
DES. J. ROBERSON

JOB NO. 11-2304

of 2 SHEETS

SEISMIC ANCHORAGE

<u>UPPER FLOOR</u>



FRONT ELEVATION

NOTES:

1. FORCES ARE DETERMINED PER 2022 CALIFORNIA BUILDING CODE AND ASCE 7-16. STRENGTH DESIGN IS USED. (EXAMPLE: SDs = 2.30, 2p = 1.0, 2p = 1.5, 2p Rp = 2.5, 2p z/h 2p 1)

HORIZONTAL FORCE (Eh) = 1.66 Wp VERTICAL FORCE (Ev) = 0.46 Wp

- 2. THIS CALCULATION ENCOMPASSES WEIGHTS AND VERTICAL C.G. POSITIONS NOT EXCEEDING VALUES SHOWN.
- 3. THIS CALCULATION WAS PREPARED WITHOUT KNOWLEDGE OF ANY SITE CONDITION. COMPATIBILITY FOR USE WITH A SITE SHALL BE EVALUATED BY THE STRUCTURAL ENGINEER OF RECORD OF THE INSTALLATION (SEOR). USE REQUIRES APPROVAL BY THE SEOR.
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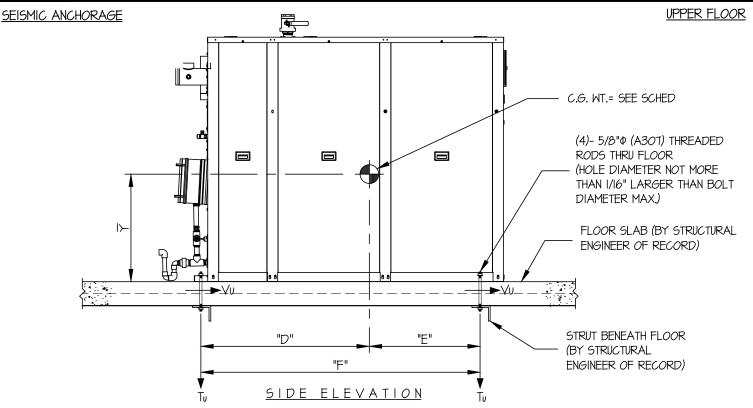
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SHEET 2

2 SHEETS



MODEL	TOTAL WEIGHT (lb.)	₹ (in.)	"A" (in.)	"B" (in.)	"C" (in.)	"D" (in.)	"E" (in.)	"F" (in.)	Tu (lb.)	Vu (lb.)
2500/3000	2329	29.64	18.31	17.69	36	32.56	28.19	60.75	1715	1331
* 3500/4000	2772	29.28	18.72	17.28	36	41.26	35.73	76.99	1944	1592

^{*} THIS UNIT IS USED IN CALCULATIONS

LOADS:

WEIGHT (W_p) = 2772 LB

HORIZONTAL FORCE (Emh) = 1.66 Wp = 4602 LB

VERTICAL FORCE (Ev) = 0.46 Wp = 1275 LB

ANCHOR SPEC: 5/8"Ø (A307) THREADED RODS

 $\phi T = 9830 \text{ LB/ANCHOR}$ (TENSION) $\phi V = 5890 \text{ LB/ANCHOR}$ (SHEAR)

ANCHOR FORCES:

TENSION (T)

$$\text{Tu maximum} = \left[\frac{4602\#(29.28'')(17.28'')}{1 \text{ bolt} (76.99'')(36'')} \times (0.3) \right] + \frac{4602\#(29.28'')(41.26'')}{1 \text{ bolt} (36'')(76.99'')} - \frac{(2772\#(0.9) - 1275\#)(17.28'')(41.26'')}{1 \text{ bolt} (36'')(76.99'')} = 1944 \text{ LB/BOLT (MAX)}$$

(HORIZ. - FRONT TO BACK)

(HORIZ. - SIDE TO SIDE)

(0.9(WEIGHT) - Ev)

SHEAR (V)

$$VU MAXIMUM = \frac{4602\#(18.72'')}{2 \text{ BOLTS}(36'')} \times (0.3) + \frac{4602\#(41.26'')}{2 \text{ BOLTS}(76.99'')} = 1592 \text{ LB/BOLT (MAX)}$$