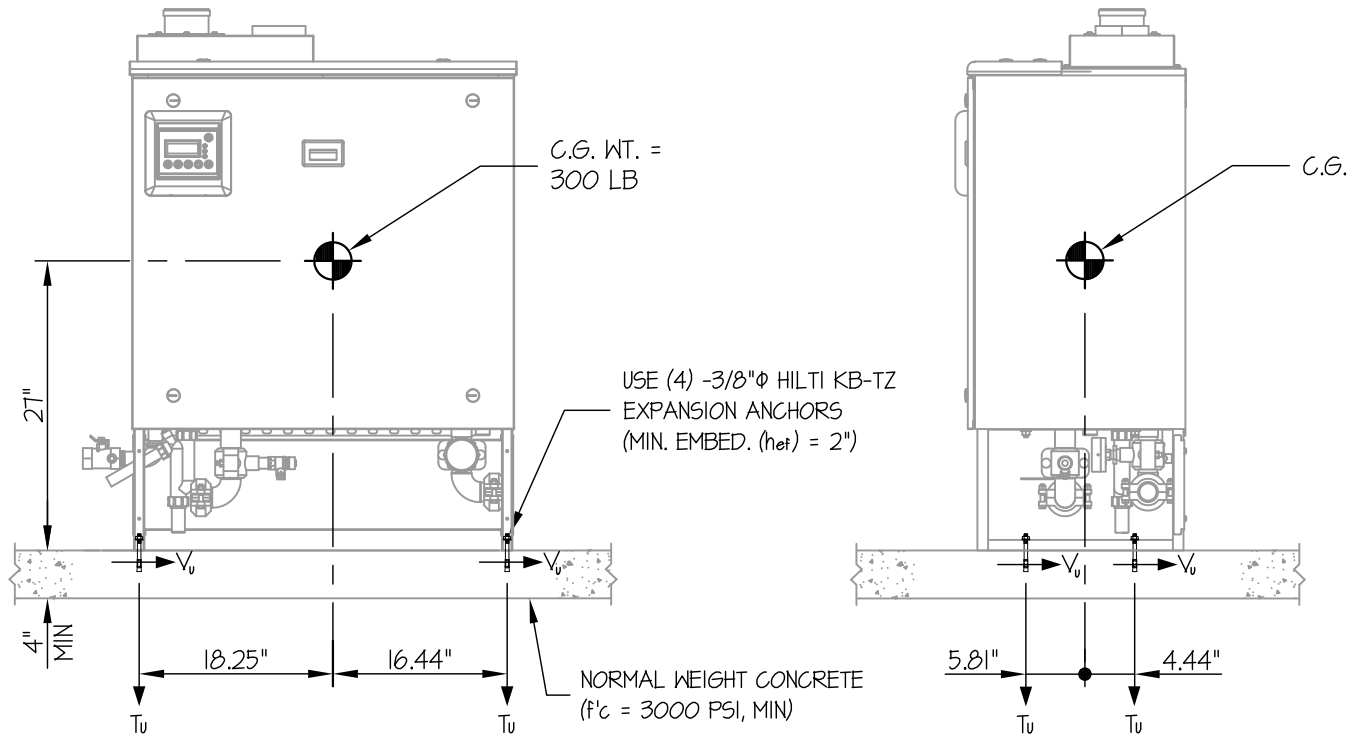


<b>PATTERSON-KELLEY CO.</b>	DES. <b>J. ROBERSON</b>	<b>SHEET</b> <b>1</b> <b>OF 1 SHEET</b>
	JOB NO. <b>11-1313</b>	
	DATE <b>2/25/13</b>	
<b>MACH MODELS CM300, CM399 &amp; CM500</b>		

SEISMIC ANCHORAGE

CONCRETE SLAB



**FRONT ELEVATION**

**SIDE ELEVATION**

LOADS: PER 2010 CALIFORNIA BUILDING CODE AND ASCE 7-05

(STRENGTH DESIGN IS USED) ( $S_{ds} = 2.00$ ,  $a_p = 1.0$ ,  $I_p = 15$ ,  $R_p = 2.5$ ,  $z/h = 0.0$ )

WEIGHT = 300 LB

HORIZONTAL FORCE ( $E_h$ ) =  $0.90 W_p = 270$  LB

VERTICAL FORCE ( $E_v$ ) =  $0.40 W_p = 120$  LB

$T_u = 367$  LB/BOLT (MAX)

$V_u = 77$  LB/BOLT (MAX)

BOLT FORCES:

TENSION (T)

$$T_{\text{MAXIMUM}} = \left[ \frac{270\#(27\")(4.44\"){}}{1_{\text{BOLT}}(34.69\")(10.25\")} \times (0.3) \right] + \frac{270\#(27\")(18.25\"){}}{1_{\text{BOLT}}(10.25\")(34.69\")} - \frac{(300\#(0.9) - 120\#(4.44\")(18.25\"))}{1_{\text{BOLT}}(10.25\")(34.69\")} = 367 \text{ LB/BOLT (MAX)}$$

(HORIZ - SIDE TO SIDE)                      (HORIZ - FRONT TO BACK)                      (WEIGHT (0.9) -  $E_v$ )

SHEAR (V)

$$V_{\text{MAXIMUM}} = \frac{270\#(5.81\"){}}{2_{\text{BOLTS}}(10.25\")} = 77 \text{ LB/BOLT (MAX)}$$

NOTE:

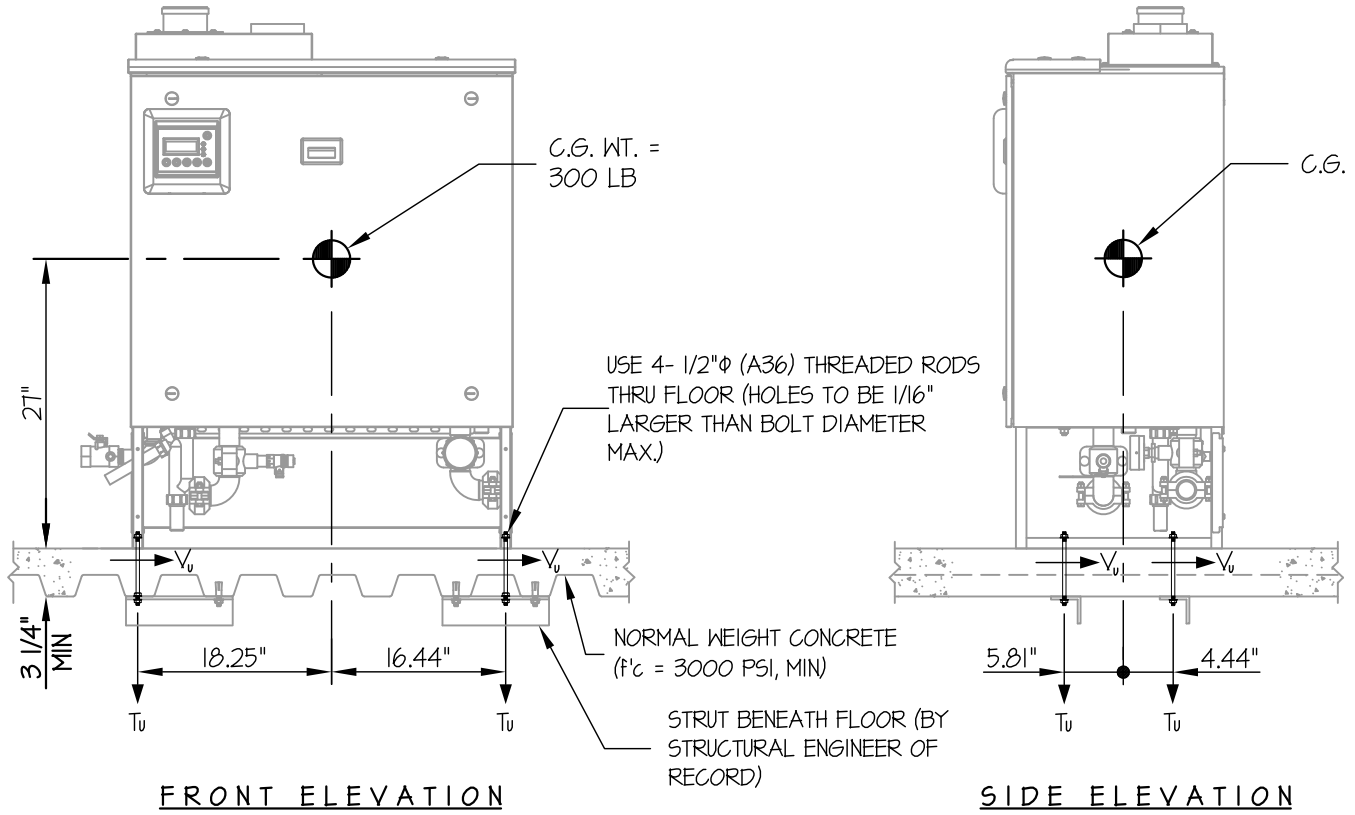
STRUCTURAL ENGINEER OF RECORD SHALL PROVIDE DESIGN OF SUPPORT STRUCTURE TO SUPPORT WEIGHTS AND FORCES SHOWN.



<b>PATTERSON-KELLEY CO.</b>	DES. <b>J. ROBERSON</b>	<b>SHEET</b> <b>1</b> <b>OF 1 SHEET</b>
	JOB NO. <b>11-1313</b>	
	DATE <b>2/25/13</b>	
<b>MACH MODELS CM300, CM399 &amp; CM500</b>		

SEISMIC ANCHORAGE

CONCRETE SLAB ON METAL DECK



**FRONT ELEVATION**

**SIDE ELEVATION**

LOADS: PER 2010 CALIFORNIA BUILDING CODE AND ASCE 7-05

(STRENGTH DESIGN IS USED) ( $S_{ds} = 2.00$ ,  $a_p = 1.0$ ,  $I_p = 1.5$ ,  $R_p = 2.5$ ,  $z/h \leq 10$ )

WEIGHT = 300 LB

HORIZONTAL FORCE ( $E_h$ ) = 144  $W_p = 432$  LB

VERTICAL FORCE ( $E_v$ ) = 0.40  $W_p = 120$  LB

$T_u = 611$  LB/BOLT (MAX)

$V_u = 122$  LB/BOLT (MAX)

BOLT FORCES:

TENSION (T)

$$T_{\text{MAXIMUM}} = \left[ \frac{432\#(27\")(5.81\"){}}{1_{\text{BOLT}}(34.69\")(10.25\")} \times (0.3) \right] + \frac{432\#(27\")(18.25\"){}}{1_{\text{BOLT}}(10.25\")(34.69\")} - \frac{(300\#(0.9) - 120\#)(5.81\")(18.25\"){}}{1_{\text{BOLT}}(10.25\")(34.69\")} = 611 \text{ LB/BOLT (MAX)}$$

( HORIZ - SIDE TO SIDE )                      ( HORIZ - FRONT TO BACK )                      ( WEIGHT (0.9) -  $E_v$  )

SHEAR (V)

$$V_{\text{MAXIMUM}} = \frac{432\#(5.81\"){}}{2_{\text{BOLTS}}(10.25\")} = 122 \text{ LB/BOLT (MAX)}$$

NOTE:

STRUCTURAL ENGINEER OF RECORD SHALL PROVIDE DESIGN OF SUPPORT STRUCTURE TO SUPPORT WEIGHTS AND FORCES SHOWN.



**PATTERSON-KELLEY CO.**

**MACH MODELS CM300, CM399 & CM500**

DES. **J. ROBERSON**

JOB NO. **11-1313**

DATE **2/25/13**

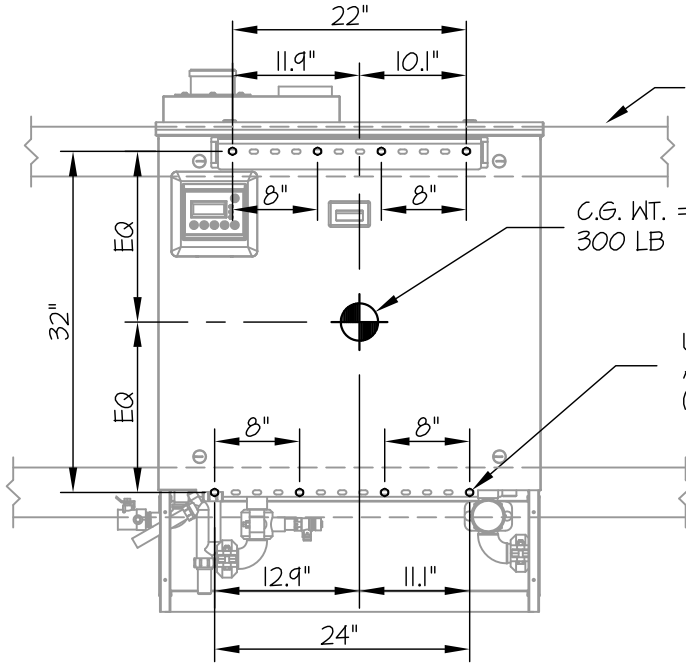
SHEET

**1**

OF **1** SHEET

SEISMIC ANCHORAGE

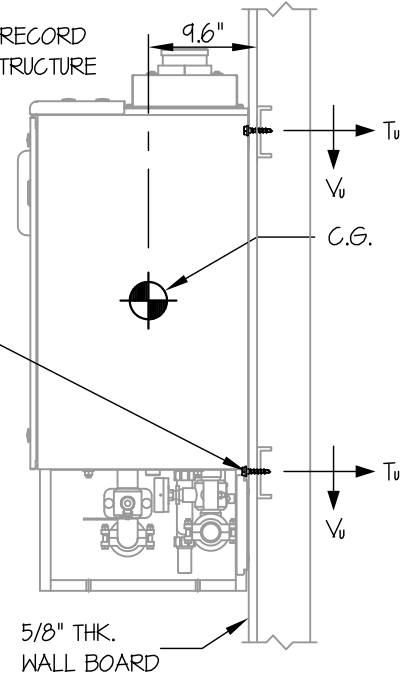
WALL MOUNTED



FRONT ELEVATION

STRUCTURAL ENGINEER OF RECORD SHALL DESIGN THE WALL STRUCTURE (16 GAGE, 50 ksi MIN.)

USE 8- 1/4"Φ TEK SCREWS AT STEEL BACKING (16 GAGE, 50 ksi MIN.)



SIDE ELEVATION

LOADS:

WEIGHT = 300 LB (MAX OPERATING WEIGHT)  
 HORIZONTAL FORCE (E<sub>h</sub>) = 1.44 W<sub>p</sub> = 432 LB  
 VERTICAL FORCE (E<sub>v</sub>) = 0.40 W<sub>p</sub> = 120 LB

BOLT FORCES:

TENSION (T)

$$T_{U \text{ VERTICAL}} = \frac{(1.2(300\#) + 120\#)(9.6\")(11.9\"){}}{2 \text{ SCREWS}(32\")(22\"){}} = 39 \text{ LB}$$

$$T_{U \text{ PARALLEL}} = \frac{432\#(9.6\"){}}{2 \text{ SCREWS}(22\"){}} = 95 \text{ LB}$$

$$T_{U \text{ PERP.}} = \frac{432\#(11.9\"){}}{4 \text{ SCREWS}(22\"){}} = 59 \text{ LB}$$

$$T_u = 39\# + 0.3(59\#) + 95\# = 152 \text{ LB/SCREW (MAX)}$$

T<sub>u</sub> = 152 LB/BOLT (MAX)  
 V<sub>u</sub> = 143 LB/BOLT (MAX)

SHEAR (V)

$$V_u = \sqrt{\left(\frac{(1.2(300\#) + 120\#)(11.9\"){}}{2 \text{ SCREWS}(22\"){}}\right)^2 + \left(\frac{432\#(11.9\"){}}{4 \text{ SCREWS}(22\"){}}\right)^2} = 143 \text{ LB/SCREW (MAX)}$$

NOTE:

STRUCTURAL ENGINEER OF RECORD SHALL PROVIDE DESIGN OF SUPPORT STRUCTURE TO SUPPORT WEIGHTS AND FORCES SHOWN.

